Valuation of Strength of Structural Steel Angle Sections, under IS Code: 800 - 1984 – A Case Study with Equal vis-a-vis Unequal Angles

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1. Introduction

Basically the structural steel is an alloy of iron, carbon-n-little parts of manganese, chrome nickel, sulfur, phosphorous-n-copper to form its rolled steel sections like angles, tees, channels, girders-n-plates which are used as beams, truss frames, columnbases, plate-girders etc to bear loads in the module of tensile-n-compressive stresses to safe the structure in deflection-n- bending. Structural steel is weather resistant and more stress bearing than normal steel. As normal specified steel is used in utensils, toys, vehicle-frames, furniture-n-watches etc., where loads and incoming varying stresses are not of structural type. While designing the steel structure two codes are usually followed viz. IS-Code: 800-1984 and Code 800-2000 which are known as elastic method and limit state method respectively.

Elastic method is simple-n-easy to design the structure, wherein stress-strain relation is considered linear till yield stress. The permissible stress is kept by dividing it by factor of safety. Factor of safety is the failure load divided by working load. But it is not true for the material. As the material can resist load after the yield appear on the fiber of steel. Hence the section becomes uneconomical.

In limit state working load is taken into account by multiplying the factor 1.5 to strengthen n serviceability of structure. The limitable strength is desired to avoid the collapse of the structure n safe the life. The permissible stresses are taken much higher than working stresses. In working stress, stresses are taken like ft=150N/mm², while ft=227 N/ mm²-n-rupture stress 295N/mm², are taken in limit state. Though the paper theme has no concern with design differentiation, however it is seem necessary to go to narrow analogous to the stresses in respective methods of design.

2. General Concept

So far the title of the paper is concern wherein the study has been made for angles of equal and unequal in size but having same cross sectional area, have been used as tension and compression members and their ends have either single riveted or double riveted.

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The strength has been calculated by using various conditional formulas and higher strength bearing condition under IS Code 800-1984, is recommended to use at construction sites where steel structures are being erected.

To make it lucid and easy & to make understandable the title to the readers it becomes necessary to illustrate by setting an example for choosing angle ISA-200*100*15 mm and ISA-150*150*15 mm having cross sectional area 4278 mm² are attached with 20mm thick gusset plate for its long leg /short leg jointed by 20mm diameter rivet shank. The permissible stress in tension has been taken 150 N/mm² for Fe-250 grade steel and corresponding stresses in compression have been taken from standard table by interpolation. It has been assumed that compression member is of discontinuous type.

3. Methodology as Tension Member

The angle members of truss are either single or double, used as tension member either side to gusset plate or one side to gusset plate. Which connection will give higher strength, either long leg connected or short leg connected to gusset plate of angle, will be treated as best in economical design for valuation.

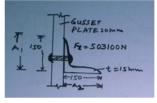
3.1 Case Study 1: Single Angle with Gusset Plate as Tension Member Riveted with 20mm Rivet Diameter

3.1.1 Connected Leg Area=(150-21.5-7.5) 15=1815 mm²

outstanding area=(150-7.5) 15 =2137.5 mm²

$$K=0.72$$

 $A_{NET} = 3354 \text{ mm}^2$
 $F_{.}=503100 \text{N}$



3.1.2 Unequal Angle

200*100*15mm whose long leg connected to 20mm thick gusset plate.

connected area=(200-21.5-7.5) 15=2565 mm² outstanding area=(100-7.5) 15=1387.5 mm² K=0.85 A_{pet}=3744.375 mm²

Ft=561656N

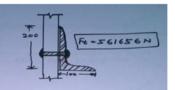


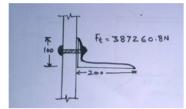
Table 1.	Comparative Stud	v Table as tension	single member

Sl.No	Angle	Leg connected	Connection	Tensile strength	Preference
1	Equal ISA-150 150 15	Any leg	single riveted	503100 N	2^{nd}
2	Unequal ISA 200 100 15	long leg	single riveted	561656 N	1^{st}
3	Unequal ISA 200 100 15	short leg	single riveted	387260.79 N	3 rd

3.1.3 Unequal Angle

200*100*15mm whose short leg is connected with 20mm thick gusset plate.

connected area=(100-21.5-7.5) 15=1065 mm² outstanding area=(200-7.5) 15= 2887.5 mm² K=0.52 A_{net}= 2581.74 mm² F=387260.79 N



3.2 Comparative Study Table as Tension Single Member

Result-single unequal angle's long leg connected to the gusset plate is showing higher strength than equal angle, whose tensile strength Ft=561656 N

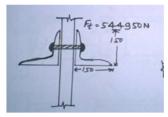
3.3 Two Angle as Tension Member

Somewhere when load is more then to restrict the stress in the tension member two members of same dimensions are used. The two angle members are connected either side or only one side to the gusset plate.

3.3.1 Case Study 2: ISA 150*150*15mm and ISA 200*100*15mm

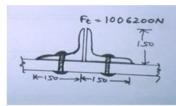
3.3.1.1 Equal Angles Either Side Connected with 20mm Thick Gusset Plate Means the Plate is Sandwiched Between Two Angles

Anet=(4278-43) 15= 3633 mm² Ft= (3633)150 N Ft=544950 N



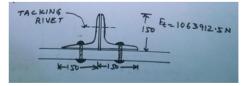
3.3.1.2 Equal Angles are One Side to the 20 mm Thick Gusset Plate without Tacking Rivet

connected area=2(150-21.5-7.5) 15= 3630 mm² outstanding area=2(150-7.5) 15=4275 mm² K=0.72 A_{net}= 3630 +0.72(4275)=6708 mm² Ft=6708(150)= 1006200 N Ft=1006200 N



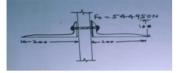
3.3.1.3 Equal Angles are One Side to 20mm Thick Gusset Plate with Tacking Rivet

Connected area= 3630 mm² outstanding area=4275 mm² A_{net}= 3630 +0.81(4275)=7092.75 mm² Ft= 7092.75(150)=1063912.5 N Ft=1063912.5 N



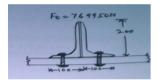
3.3.1.4 Unequal Angles Short Leg is Connected with 20mm Thick Gusset Plate on its Either Side

A_{net}=3633mm² Ft= (3633) 150= 544950 N Ft=544950N



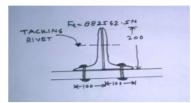
3.3.1.5 Unequal Angles Short Leg is Connected with 20mm Thick Gusset Plate on its One Side without Tacking Rivet

Connected area= 2(100-21.5-7.5) 15= 2130 mm² outstanding area= 2(200-7.5) 15= 5775 mm² K=0.52 A_{net} = 2130 + 0.52(5775) =5133 mm² Ft= 5133 (150)= 769950 N Ft=769950 N



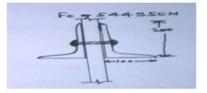
3.3.1.6 Unequal Angles Short Leg is Connected with 20mm Thick Gusset Plate on its One Side with Tacking Rivet

Connected area= 2130 mm² outstanding area= 5775 mm² K=0.65 $A_{net}= 2130+0.65(5775)= 5883.75 mm²$ Ft= 5883.75 (150) = 882562.5 mm² $F_t=882562.5 N$



3.3.1.7 Unequal Angles Long Leg is Connected with 20mm Thick Gusset Plate on its Either Side

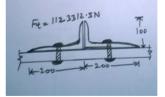
A_{net}= 3633 mm² Ft= 3633 (150)= 544950 Ft=544950 N



3.3.1.8 Unequal Angles Long Leg is Connected with 20mm Thick Gusset Plate on its One Side without Tacking Rivet

Connected area=2(200-21.5-7.5) 15= 5130 mm² Outstanding area= 2(100-7.5) 15=2775 mm² K=0.85

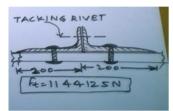
A_{net} = 5130 +0.85 (2775)= 7488.75 mm² Ft= 7448.75 (150)= 1123312.5 N F.=1123312.5 N



3.3.1.9 Unequal Angles Long Leg is Connected with 20mm Thick Gusset Plate on its One Side with Tacking Rivet

Connected area=2(200-21.5-7.5)15=5130 mm² outstanding area=2(100-7.5)15=2775mm² K=0.90 A_{net}= 5130+0.90(2775)=7627.5 mm² Ft=7627.5 (150)=1144125 N

Ft=1144125N



3.4 Comparative Table for Tensile Strength of Two Angles Connected with 20mm Thick Gusset Plate on its Various Positions

Result- Double unequal angles long leg connected with one side of gusset plate without tacking

rivet is giving higher strength in tension whose Ft=1144125N (Table 2).

3.5 Discontinuous Compression Member

The members are either single equal or un-equal and double equal single or unequal and their legs are connected with 20mm thick gusset plate on various conditions with ends rivet either single riveted or double riveted with a length of 2000mm have been assumed and illustrated to get the result, for the theme of the paper.

3.5.1 Equal Single Member Connected to the Gusset Plate with Ends Single Riveted

 $r_{vv} = 29.2, l = 2000 \text{ mm}, l/r_{vv} = 2000/29.2 = 68.5$ stress at l/r_{vv} 68.5=113.5 N/mm² Fc=4278(113.5)(0.8) = 388442.4 N F_=388442.4 N

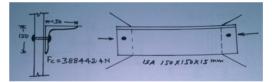
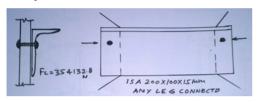


 Table 2.
 Comparative table for tensile strength of two angles connected with 20mm thick gusset plate on its various positions

Sl.No.	Double Angle	Condition to gusset plate	Connection	Tensile Strength	Preference
1	ISA 150 150 15	Either side	Any Leg	544950N	Same
2	ISA 200 100 15	Either side	Long Leg	544950N	Same
3	ISA 200 100 15	Either side	Short Leg	544950N	Same
1	150 150 15	one side without tacking	Any Leg	1006200 N	2 nd
2	200 100 15	one side without tacking	Long Leg	1123312 N	1^{st}
3	200 100 15	one side without tacking	Short Leg	769950 N	$3^{\rm rd}$
1	150 150 15	one side without tacking	Any Leg	1063912.5 N	2^{nd}
2	200 100 15	one side without tacking	Long Leg	1144125 N	1^{st}
3	200 100 15	one side without tacking	Short Leg	882562 N	3 rd

3.5.2 Unequal Single Angle Any Leg is Connected with Gusset Plate with Ends Single Riveted

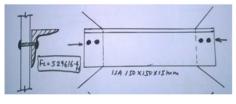
 $r_{yy} = 26.4, l = 2000 \text{ mm }, l/r_{yy} = 2000/26.4 = 75.75$ stress at l/r_{yy}75.75 = 103.475 N/mm² Fc= 4278(103.475)(0.8) = 354132.8 N/mm² F_= 354132.8 N



3.5.3 Equal Angle Single with Ends Double Riveted

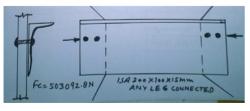
 r_{vv} =29.2, l=0.85(2000)=1700mm, l/ r_{vv} =1700/29.2 =58.2

stress at l/r_{vv} 58.2 =123.8 N/mm² Fc= 4278(123.8)= 529616.4 N/mm² F_c=529616.4 N



3.5.4 Unequal Single Angle any Leg Connected with Ends Double Riveted

 r_{yy} =26.4, l=1700mm, l/ r_{yy} =1700/26.4 =64.4 stress at l/ r_{yy} 64.4 = 117.6N /mm² Fc=117.6(4278)=503092 .8 N F_{c} =503092.8 N

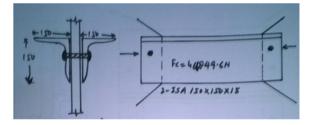


3.6 Discontinuous Compression Member Double Equal and Double Unequal

Result=Equal angle single with ends double riveted is more strength full compression member than unequal angle, whose F_c =529616.4N connected with gusset plate under various positions may be there and they are having different strength. The higher resulting strength will show the good valuated strength steel angle section.

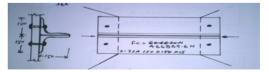
3.6.1 Equal Angles either Side of Gusset Plate with Single Riveted

$$\begin{split} l=&2000\,\text{mm},\,I_{xx}=&2(8968000)=&17936000\,\text{mm}^4\\ I_{yy}=&2(8968000)+&2(4278)(52.6)^2\\ =&41608398.56\,\,\text{mm}^4\\ I_{min}=&17936000\,\text{mm}^4\\ rxx=&45.8,\,l/rxx=&2000/45.8\ =&43.7\\ stress at \,l/r_{xx}\,43.7=&136.4\,\,\text{N/mm}^2\\ Fc=&4278(136.4)(0.8)=&466849.584\text{N}\\ Fc=&466849.584\,\,\text{N} \end{split}$$



3.6.2 Equal Angles are One Side of Gusset Plate with Single Riveted

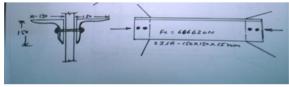
Things will be same as in 3.6-1 Fc=466849.584 N



Sl.No.	Angle Single	Leg Connected	End Condition	Compressive Strength	Preferance
1	150 150 15	Any Leg	single riveted	388442.4 N	3 rd
2	200 100 15	Any Leg	single riveted	354132.8 N	4^{th}
3	150 150 15	Any Leg	single riveted	529616.4 N	1^{st}
4	200 100 15	Any Leg	single riveted	503092.8 N	2^{nd}

3.6.3 Equal Angles either Side to Gusset Plate, Ends Double Riveted

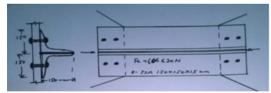
l=0.85(2000)=1700mm, r_{xx}=45.8, l/r_{xx}=1700/45.8=37, stress at $l/r_{...}37 = 141.8$ N/mm² Fc=4278(141.8)=606620 N Fc=606620 N



3.6.4 Equal Angles One Side to Gusset Plate, Ends Double Riveted

This will be same as in 3.6-3

Fc=606620 N

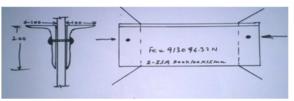


3.6.5 Unequal Angle Long Leg, Connected either Side to the Gusset Plate, Ends Single Riveted

 $I_{yy} = 2(17500000) = 35000000 \text{ mm}^4$

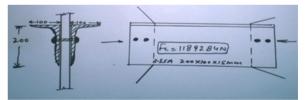
 $I_{vv} = 2(2981000) + 2(4278)(32.2)^2 = 14833203 \text{ mm}^4$ I_{min}=14833203 mm⁴ r_{min}=41.63 $l/r_{min} = 2000/41.63 = 48$ stress al l/r_{min}48=133.4 N/mm²

Fc=2(4278)(133.4)=913096.32 N Fc=913096.32N



3.6.6 Unequal Angle Long Leg, Connected either Side to the Gusset Plate, Ends -**Double Riveted**

l=1750mm, r_{min}=41.63 l/r_{min} =1750/41.63=40.83 or 40 stress at l/r_{min} 40=139 N/mm² Fc=2(4278)(139)=1189284 N Fc=1189284 N



3.6.7 Unequal Angles Short Leg Connected either Side to Gusset Plate Ends Single Riveted

lxx=2(2981000)=5962000 mm⁴

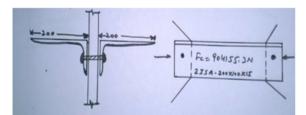
Iyy=2(17500000)+2(4278)(81.8)2=92250249.44 mm4

I_{min}=5962000mm⁴ $r_{min} = 26.4$ l/r_{min}=2000/26.4=75.75 Valuation of Strength of Structural.....

Table 4	Comparative table for double angles as compression member under various positions					
Sl.no.	Double	Leg	End	Gusset plate Condition	Compressive-	Preference
	Angle	Connected	Condition		strength	
1	150 150 15	Any Leg	single riveted	Angle either side of plate	466849.6 N	8 th
2	150 150 15	Any Leg	single riveted	Angle one side of plate	466849.6 N	8^{th}
3	150 150 15	Any Leg	Double riveted	Angle one side of plate	597208.8 N	7^{th}
4	150 150 15	Any Leg	Double riveted	Angle either side of plate	597208.8 N	7^{th}
5	200 100 15	Long Leg	single riveted	Angle either side of plate	913096.3 N	4^{th}
6	200 100 15	Long Leg	Double riveted	Angle either side of plate	1189284 N	1^{st}
7	200 100 15	Short Leg	single riveted	Angle either side of plate	904555.3 N	5^{th}
8	200 100 15	Long Leg	single riveted	Angle one side of plate	904555.3 N	5^{th}
9	200 100 15	Long Leg	Double riveted	Angle one side of plate	999600 N	3^{rd}
10	200 100 15	Short Leg	Double riveted	Angle either side of plate	999600 N	$3^{\rm rd}$
11	200 100 15	Short Leg	single riveted	Angle one side of plate	848160 N	6^{th}
12	200 100 15	Short Leg	Double riveted	Angle either side of plate	1132875 N	2^{nd}

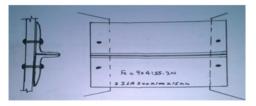
Table 4. Comparative table for double angles as compression member under various positions

stress for l/r_{min} 75.75=105.675 N/mm² Fc=2(4278)(105.675)=904155.3N Fc=904155.3 N



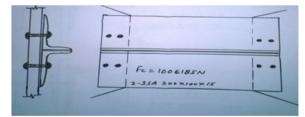
3.6.8 Unequal Angles Long Leg Connected One Side to the Gusset Plate, Ends Single Riveted

The strength will be same as in 3.6-7 Fc=904155.3 N



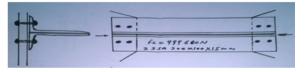
3.6.9 Unequal Angles Long Leg Connected One Side to the Gusset Plate Ends Double Riveted

l=1700 mm, r_{min}=26.4, l/r_{min}=1700/26.4=64.4 stress at l/r_{min}64.4=117.6N/mm² Fc=2(4278)(117.6)=1006185 N Fc=1006185N



3.6.10 Unequal Angles Short Leg Connected either Side to the Gusset Plate Ends Double Riveted

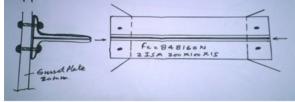
Fc=999600N



3.6.11 Unequal Angles Short Leg one Side to Gusset Plate Ends Single Riveted

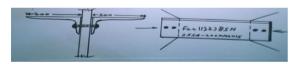
 $I_{xx} = 3500000 \text{ mm}^4$ $I_{yy} = 1017782 \text{ mm}^4$ $r_{min} = 34.5, 1/r_{min} = 2000/34.5 = 58$ stress at 1/r_min 58=124N/mm² Fc=2(4278)(124)=848160





3.6.12 Unequal Angles Short Leg either Side of Gusset Plate, Ends Double Riveted

Fc=1132875 N



4. Result and Discussions

The study reveals that the angles bearing same crosssectional area of either equal or unequal angles, whether they are single or double, while using as tension member, unequal angles-long leg connected with gusset plate will show higher strength. Long leg connected one side to gusset plate with tacking rivet as tension member has shown highest strength than other conditions while using double angles So far the compression member is concerned, single equal angle has shown good strength with ends double riveted, than unequal angle. If double angles are used then unequal angles long leg connection, either side to gusset plate, with ends double riveting has been showing the highest compressive strength than only side connection to gusset plate even equal angles.

Hence it is finally discussed with this paper and recommended to use single unequal angle with its long leg connected to gusset plate and using it as tension member is preferable rather than equal angle rather than unequal short leg connected as constrain. Also if double angles are being used as tension member then unequal angles long leg connected to gusset plate to its one side with tacking rivet will be showing highest tensile strength than without tacking rivet. If constrain then equal angles one side to gusset plate plus tacking rivet and then without tacking rivet may be used. Short leg connection is not recommended to use in structures.

As compression member single equal angle with ends double riveted is recommended to use in place of unequal angle. It is too recommended that if double angles are being used as compression member than long leg connected either side to gusset plate is having highest strength rather than one side to gusset plate of long leg connected. Equal angles are not recommended to use as compression member.

5. Reference

1. Varshaney MK. Book design of steel and masonry structures.